

# 矩阵位移法

# 1 概述

矩阵位移法是以结构位移为基本未知量，借助矩阵进行分析，并用计算机解决各种杆系结构受力、变形等计算的方法。

理论基础：位移法

分析工具：矩阵

计算手段：计算机

# 基本思想:

•化整为零 ----- 结构离散化

将结构拆成杆件,杆件称作**单元**.

单元的连接点称作**结点**.

对单元和结点编码.

基本未知量:结点位移

•单元分析

单元杆端力  $\Leftrightarrow$  单元杆端位移

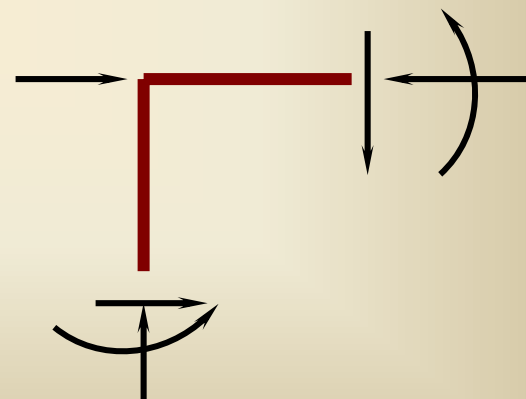
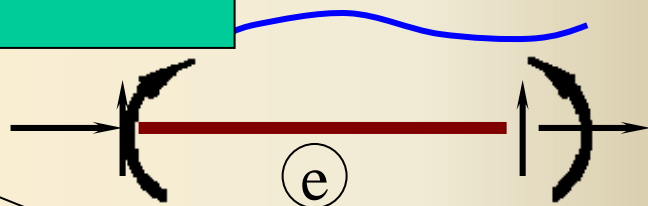
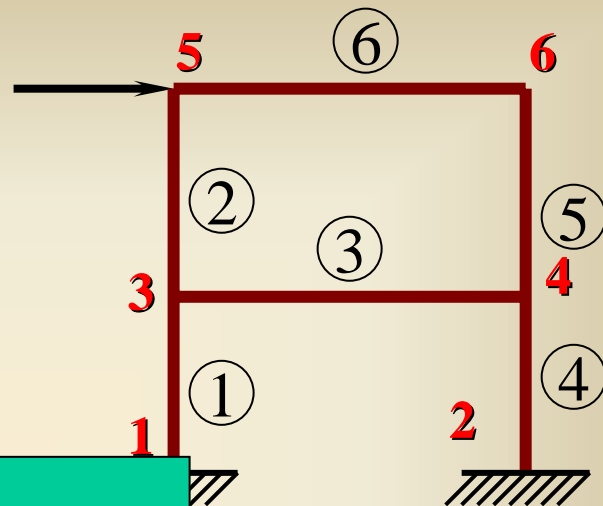
•集零为整 ----- 整体分析

结点外力  $\Leftrightarrow$  单元杆端力

结点外力  $\Leftrightarrow$  单元杆端位移

(杆端位移=结点位移)

结点外力  $\Leftrightarrow$  结点位移



## 2 矩阵位移法解连续梁

### 一.离散化

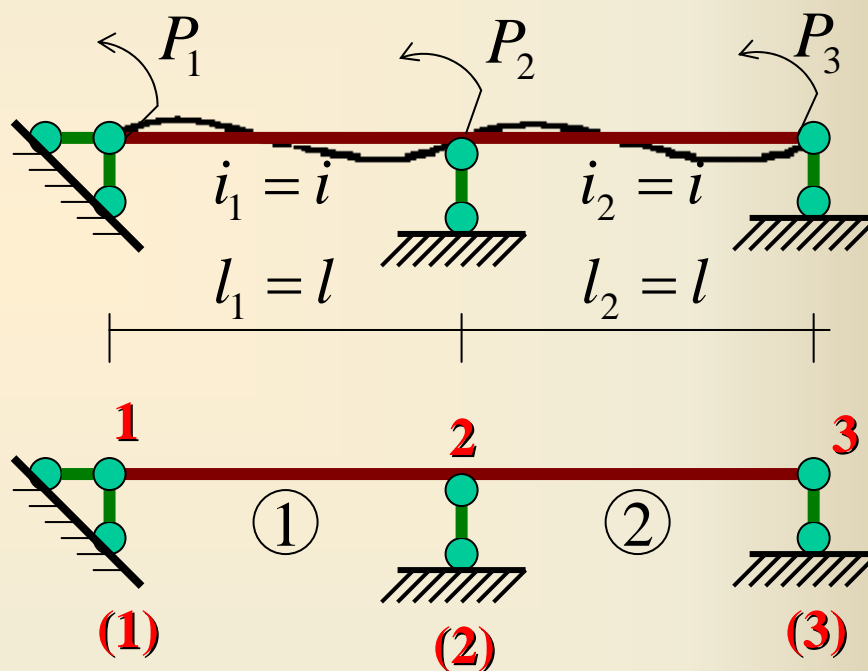
结点位移逆时针为整,  
结点力逆时针为整.

① ② ----单元编码

1,2,3 ----结点编码

(1),(2),(3) ----结点位移编码

----整体编码



## 二.单元分析

1,2----局部编码

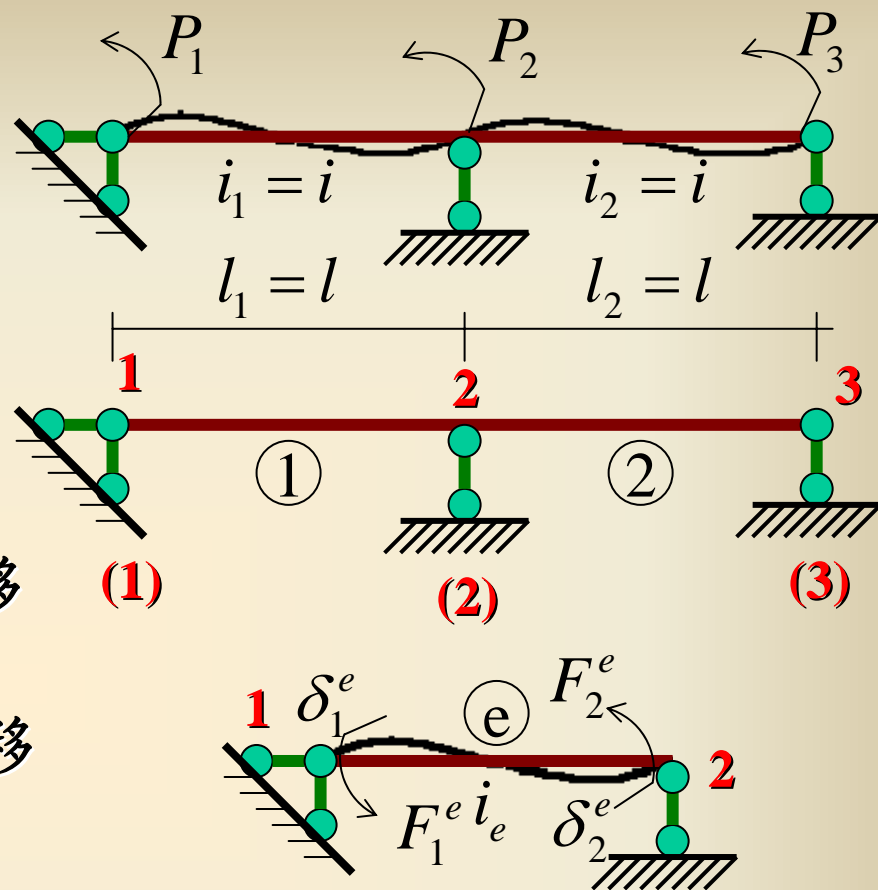
$$\{F\}^e = \begin{Bmatrix} F_1^e \\ F_2^e \end{Bmatrix} \text{----单元杆端力}$$

$$\{\delta\}^e = \begin{Bmatrix} \delta_1^e \\ \delta_2^e \end{Bmatrix} \text{----单元杆端位移}$$

单元杆端力和单元杆端位移  
逆时针为正.

单元分析的目的:

建立单元杆端力和单元杆端位移的关系.



## 二.单元分析

1,2----局部编码

$$\{F\}^e = \begin{Bmatrix} F_1^e \\ F_2^e \end{Bmatrix} \text{----单元杆端力}$$

$$\{\delta\}^e = \begin{Bmatrix} \delta_1^e \\ \delta_2^e \end{Bmatrix} \text{----单元杆端位移}$$

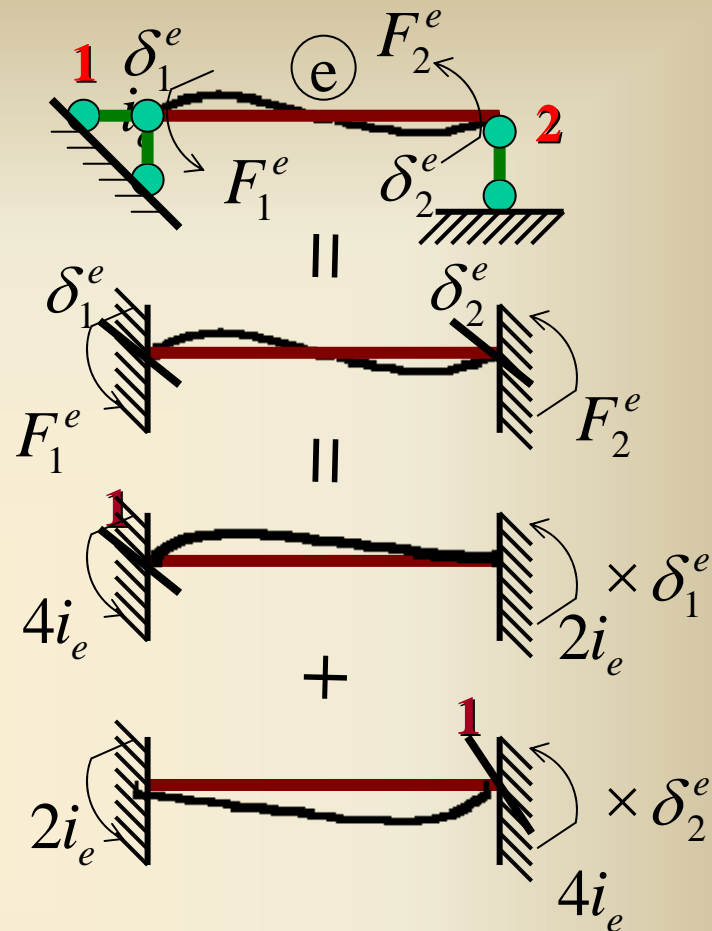
单元分析的目的:

建立单元杆端力和单元杆端位移的关系.

$$\begin{aligned} F_1^e &= 4i_e \delta_1^e + 2i_e \delta_2^e \\ F_2^e &= 2i_e \delta_1^e + 4i_e \delta_2^e \end{aligned} \quad \begin{Bmatrix} F_1^e \\ F_2^e \end{Bmatrix} = \begin{bmatrix} 4i_e & 2i_e \\ 2i_e & 4i_e \end{bmatrix} \begin{Bmatrix} \delta_1^e \\ \delta_2^e \end{Bmatrix}$$

简记为  $\{F\}^e = [k]^e \{\delta\}^e$  ---单元刚度方程

其中  $[k]^e$  称作单元刚度矩阵(简称作单刚)



## 二.单元分析

单元刚度矩阵中元素的物理意义

$$[k]^e = \begin{bmatrix} k_{11}^e & k_{12}^e \\ k_{21}^e & k_{22}^e \end{bmatrix} = \begin{bmatrix} 4i_e & 2i_e \\ 2i_e & 4i_e \end{bmatrix}$$

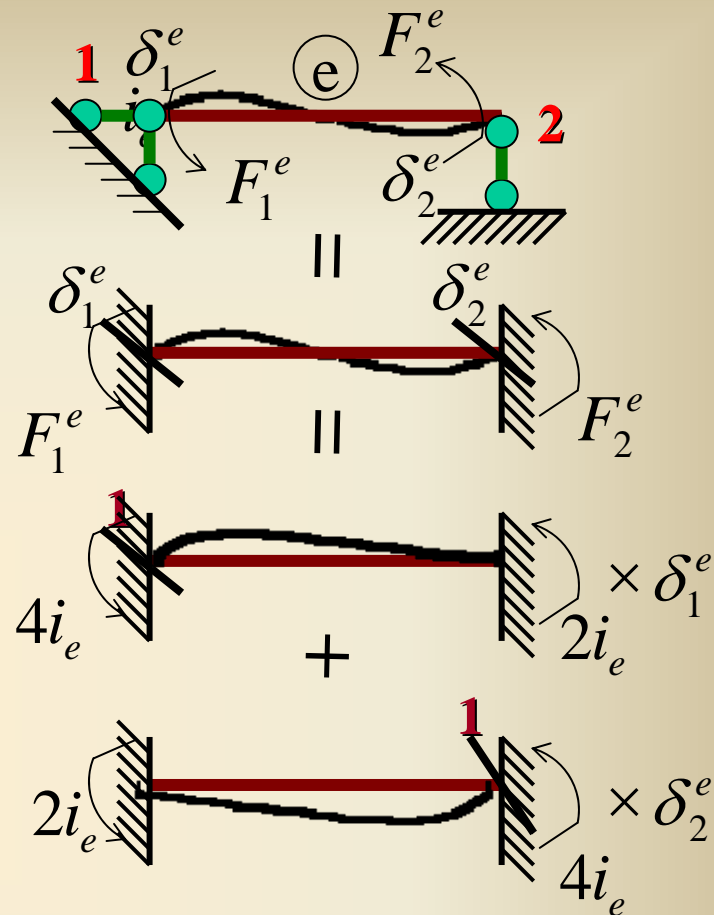
$k_{ij}^e$  --- 发生  $\delta_j^e = 1, \delta_i^e = 0$  位移时在  $i$  端所需加的杆端力。

单元刚度矩阵性质: 对称矩阵

$$\begin{aligned} F_1^e &= 4i_e \delta_1^e + 2i_e \delta_2^e \\ F_2^e &= 2i_e \delta_1^e + 4i_e \delta_2^e \end{aligned} \quad \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix}^e = \begin{bmatrix} 4i_e & 2i_e \\ 2i_e & 4i_e \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \end{Bmatrix}^e$$

简记为  $\{F\}^e = [k]^e \{\delta\}^e$  --- 单元刚度方程

其中  $[k]^e$  称作单元刚度矩阵(简称作单刚)



### 三. 整体分析

整体分析的目的:

建立结点力与结点位移的关系.

$$P_1 = k_{11}\delta_1 + k_{21}\delta_2 + k_{31}\delta_3$$

$$P_2 = k_{21}\delta_1 + k_{22}\delta_2 + k_{32}\delta_3$$

$$P_3 = k_{31}\delta_1 + k_{32}\delta_2 + k_{33}\delta_3$$

$$\begin{Bmatrix} P_1 \\ P_2 \\ P_3 \end{Bmatrix} = \begin{bmatrix} k_{11} & k_{12} & k_{13} \\ k_{21} & k_{22} & k_{23} \\ k_{31} & k_{32} & k_{33} \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix}$$

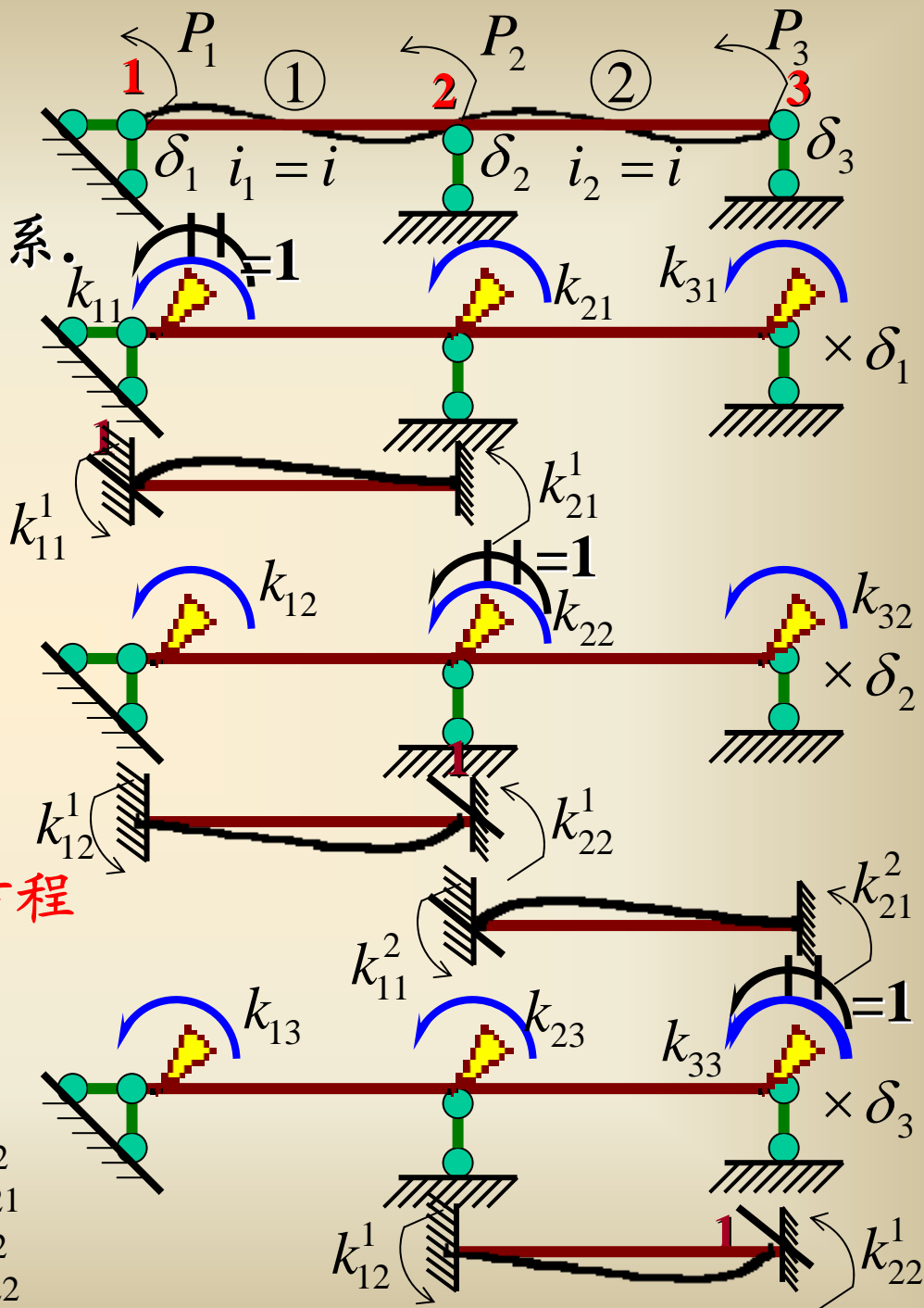
简记为  $\{P\} = [k]\{\Delta\}$  --- 结构刚度方程

$[k]$  -- 结构刚度矩阵(总刚)

$$k_{11} = k_{11}^1 \quad k_{21} = k_{21}^1 \quad k_{31} = 0$$

$$k_{12} = k_{12}^1 \quad k_{22} = k_{22}^1 + k_{11}^2 \quad k_{32} = k_{21}^2$$

$$k_{13} = 0 \quad k_{23} = k_{12}^2 \quad k_{33} = k_{22}^2$$





# 单元刚度矩阵中元素的物理意义

$$[k] = \begin{bmatrix} k_{11} & k_{12} & k_{13} \\ k_{21} & k_{22} & k_{23} \\ k_{31} & k_{32} & k_{33} \end{bmatrix}$$

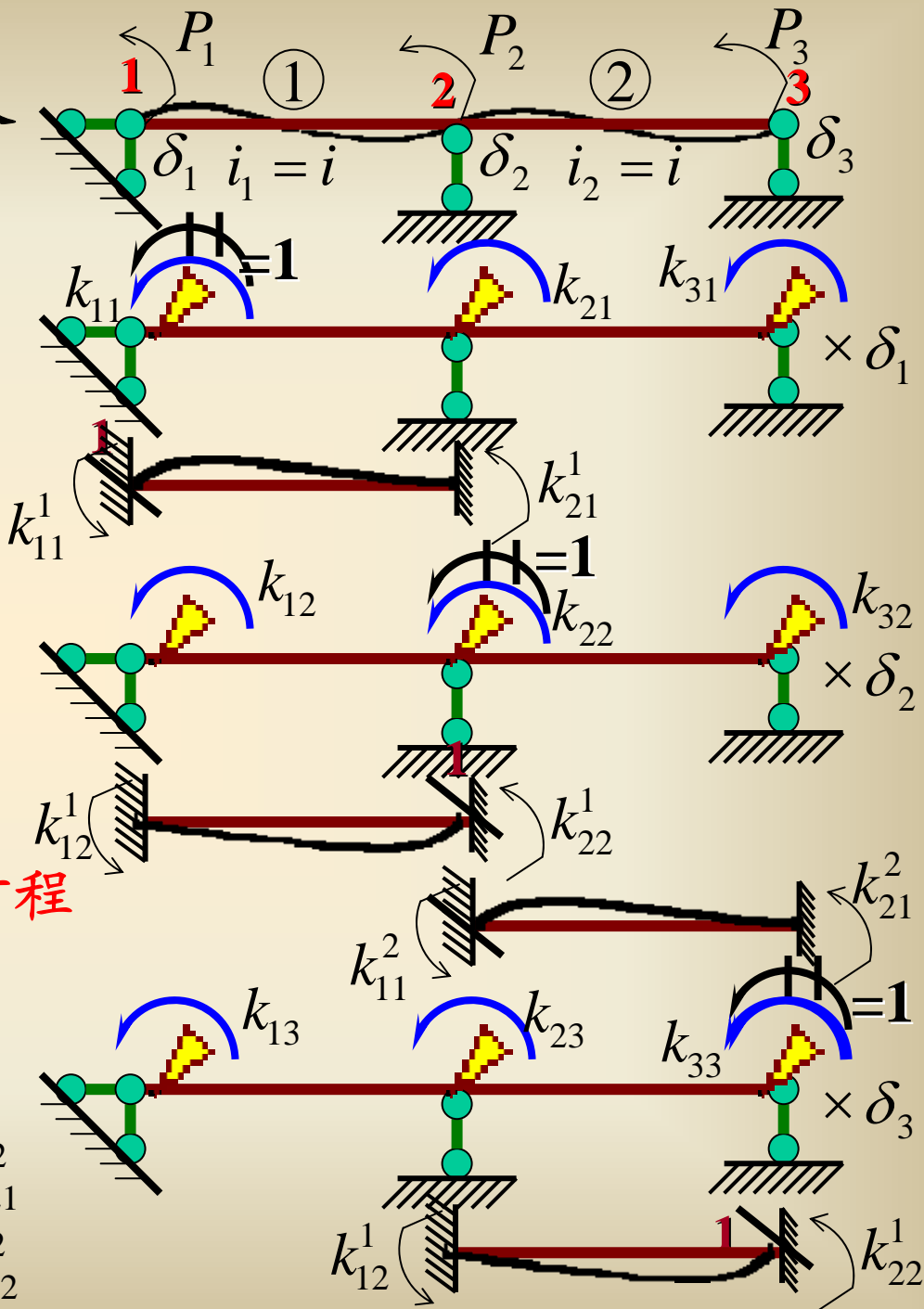
$k_{ij}$  --- 发生  $\delta_j = 1$ , 其它结点位移为零位移时在  $i$  结点所加的结点力。

结构刚度矩阵性质: 对称矩阵

简记为  $\{P\} = [k]\{\Delta\}$  --- 结构刚度方程

$[k]$  -- 结构刚度矩阵(总刚)

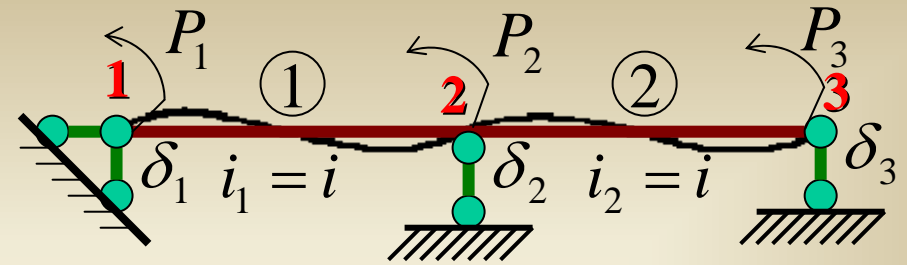
$$\begin{aligned} k_{11} &= k_{11}^1 & k_{21} &= k_{21}^1 & k_{31} &= 0 \\ k_{12} &= k_{12}^1 & k_{22} &= k_{22}^1 + k_{11}^2 & k_{32} &= k_{21}^2 \\ k_{13} &= 0 & k_{23} &= k_{12}^2 & k_{33} &= k_{22}^2 \end{aligned}$$



# 单元刚度矩阵中元素的物理意义

$$[k] = \begin{bmatrix} k_{11} & k_{12} & k_{13} \\ k_{21} & k_{22} & k_{23} \\ k_{31} & k_{32} & k_{33} \end{bmatrix}$$

$k_{ij}$  --- 发生  $\delta_j = 1$ , 其它结点位移为零位移时在  $i$  结点所加的结点力。



总刚的形成方法 --- “对号入座”

$$[k]^1 = \begin{bmatrix} \cancel{\frac{1}{1}} & \cancel{\frac{2}{2}} \\ k_{11}^1 & k_{12}^1 \\ k_{21}^1 & k_{22}^1 \end{bmatrix} \begin{matrix} \bar{1} & 1 \\ \bar{2} & 2 \end{matrix}$$

结构刚度矩阵性质: 对称矩阵

简记为  $\{P\} = [k]\{\Delta\}$  --- 结构刚度方程

$$[k] = \begin{bmatrix} k_{11}^1 & k_{12}^1 & 0 \\ k_{21}^1 & k_{22}^1 + k_{11}^2 & k_{12}^2 \\ 0 & k_{21}^2 & k_{22}^2 \end{bmatrix} \begin{matrix} 1 \\ 2 \\ 3 \end{matrix}$$

$[k]$  -- 结构刚度矩阵(总刚)

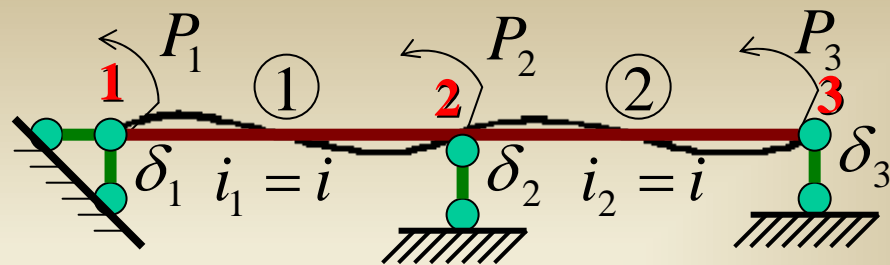
$$\begin{aligned} k_{11} &= k_{11}^1 & k_{21} &= k_{21}^1 & k_{31} &= 0 \\ k_{12} &= k_{12}^1 & k_{22} &= k_{22}^1 + k_{11}^2 & k_{32} &= k_{21}^2 \\ k_{13} &= 0 & k_{23} &= k_{12}^2 & k_{33} &= k_{22}^2 \end{aligned}$$

$$[k]^2 = \begin{bmatrix} \cancel{\frac{2}{1}} & \cancel{\frac{3}{2}} \\ k_{11}^2 & k_{12}^2 \\ k_{21}^2 & k_{22}^2 \end{bmatrix} \begin{matrix} \bar{1} & 2 \\ \bar{2} & 3 \end{matrix}$$

#### 四. 计算杆端力

$\{P\} = [k]\{\Delta\}$  计算结点位移

$\{F\}^e = [k]^e \{\delta\}^e$  计算杆端力



# 四. 计算杆端力

$\{P\} = [k]\{\Delta\}$  计算结点位移

$\{F\}^e = [k]^e\{\delta\}^e$  计算杆端力

例：计算图示梁，作弯矩图

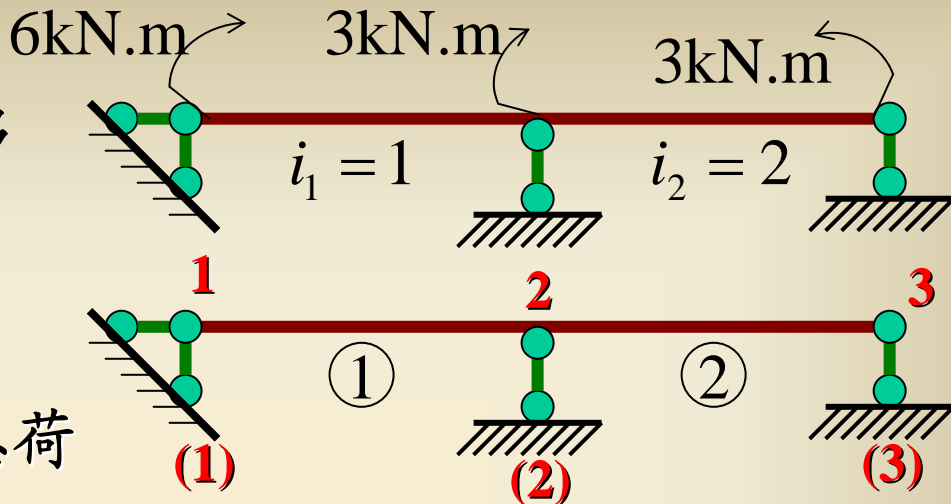
解：1. 离散化 2. 计算总刚，总荷

$$[k]^1 = \begin{bmatrix} \cancel{1} & \cancel{2} \\ 4 & 2 \\ 2 & 4 \end{bmatrix} \begin{matrix} 1 \\ 2 \end{matrix} \quad [k]^2 = \begin{bmatrix} \cancel{2} & \cancel{3} \\ 8 & 4 \\ 4 & 8 \end{bmatrix} \begin{matrix} 1 \\ 2 \end{matrix}$$

$$[k] = \begin{bmatrix} 4 & 2 & 0 \\ 2 & 12 & 4 \\ 0 & 4 & 8 \end{bmatrix} \quad \{P\} = \begin{Bmatrix} -6 \\ -3 \\ 3 \end{Bmatrix}$$

3. 解方程，求位移

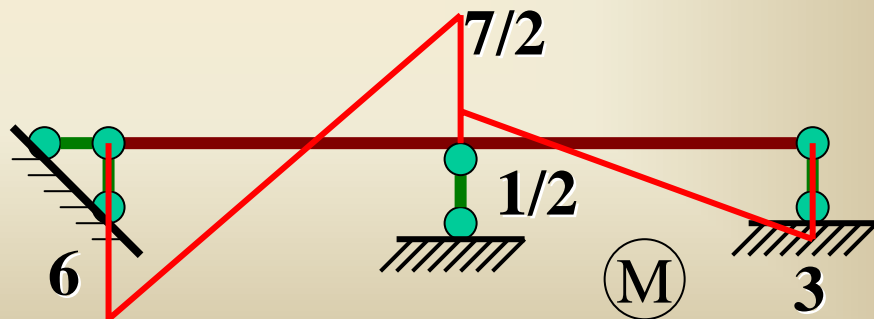
$$\{P\} = [k]\{\Delta\} \quad \{\Delta\} = \begin{Bmatrix} -17/12 \\ -1/6 \\ 11/24 \end{Bmatrix}$$



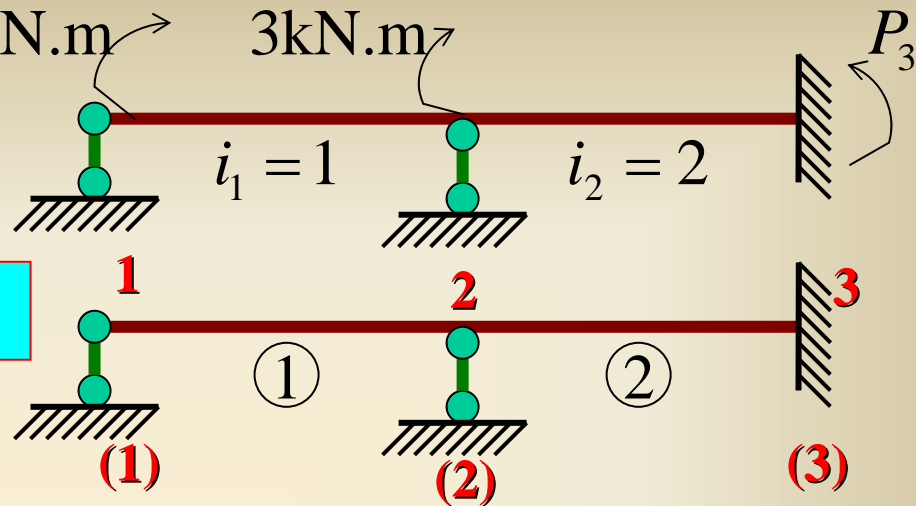
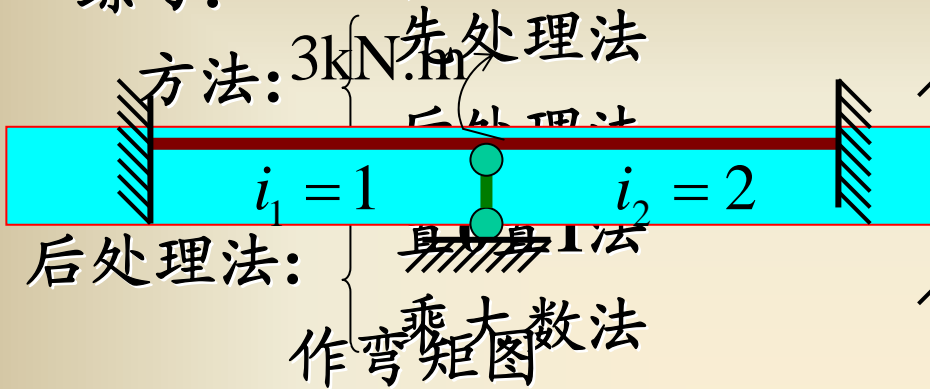
4. 求杆端力

$$\{F\}^1 = \begin{bmatrix} 4 & 2 \\ 2 & 4 \end{bmatrix} \begin{Bmatrix} -17/12 \\ -1/6 \end{Bmatrix} = \begin{Bmatrix} -6 \\ -7/2 \end{Bmatrix}$$

$$\{F\}^2 = \begin{bmatrix} 8 & 4 \\ 4 & 8 \end{bmatrix} \begin{Bmatrix} -1/6 \\ 11/24 \end{Bmatrix} = \begin{Bmatrix} 1/2 \\ 3 \end{Bmatrix}$$



# 乘大数法处理边界条件



## (1) 置0置1法

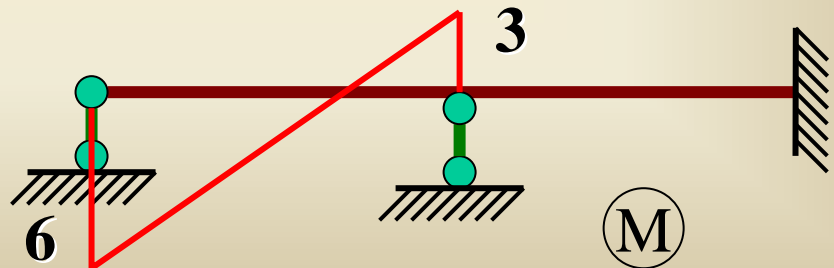
$$\begin{bmatrix} 4 & 2 & 0 & 0 \\ 2 & 12 & 0 & 4 \\ 0 & 4 & 8 & 0 \\ 0 & 0 & 1 & 0 \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \\ 0 \end{Bmatrix} = \begin{Bmatrix} -6 \\ -3 \\ -P_3 \\ 0 \end{Bmatrix}$$

$\delta_3 = 0$

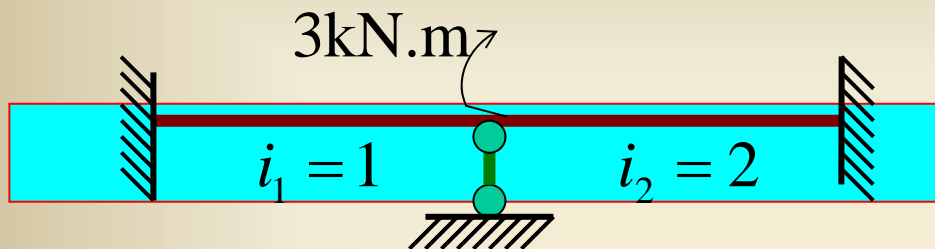
$$\begin{bmatrix} 4 & 2 & 0 \\ 2 & 12 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix} = \begin{Bmatrix} -6 \\ -3 \\ 0 \end{Bmatrix}$$

$$\{\Delta\} = \begin{Bmatrix} -3/2 \\ 0 \\ 0 \end{Bmatrix} \quad \{F\}^2 = \begin{bmatrix} 8 & 4 \\ 4 & 8 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$$

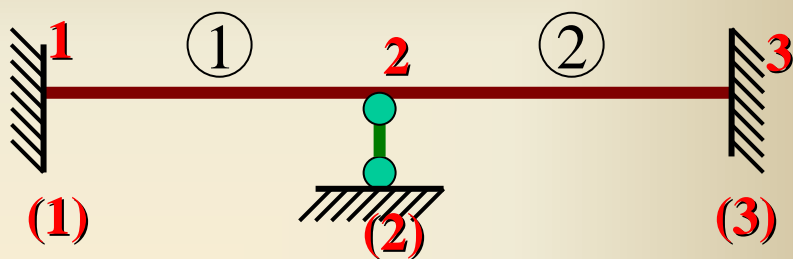
$$\{F\}^1 = \begin{bmatrix} 4 & 2 \\ 2 & 4 \end{bmatrix} \begin{Bmatrix} -3/2 \\ 0 \end{Bmatrix} = \begin{Bmatrix} -6 \\ -3 \end{Bmatrix}$$



练习:



作弯矩图



$$\begin{bmatrix} 4 & 2 & 0 \\ 2 & 12 & 4 \\ 0 & 4 & 8 \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix} = \begin{Bmatrix} -P_1 \\ -3 \\ P_3 \end{Bmatrix}$$

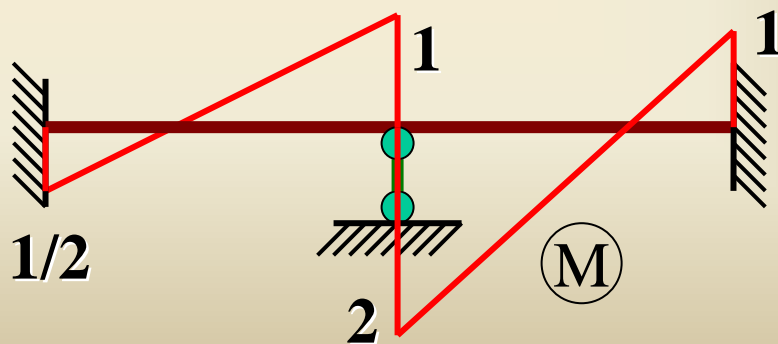
$$\begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix} = \begin{Bmatrix} 0 \\ -1/4 \\ 0 \end{Bmatrix}$$

$$\{F\}^1 = \begin{bmatrix} 4 & 2 \\ 2 & 4 \end{bmatrix} \begin{Bmatrix} 0 \\ -1/4 \end{Bmatrix} = \begin{Bmatrix} -1/2 \\ -1 \end{Bmatrix}$$

$$\{F\}^2 = \begin{bmatrix} 8 & 4 \\ 4 & 8 \end{bmatrix} \begin{Bmatrix} -1/4 \\ 0 \end{Bmatrix} = \begin{Bmatrix} -2 \\ -1 \end{Bmatrix}$$

$$\delta_1 = \delta_3 = 0$$

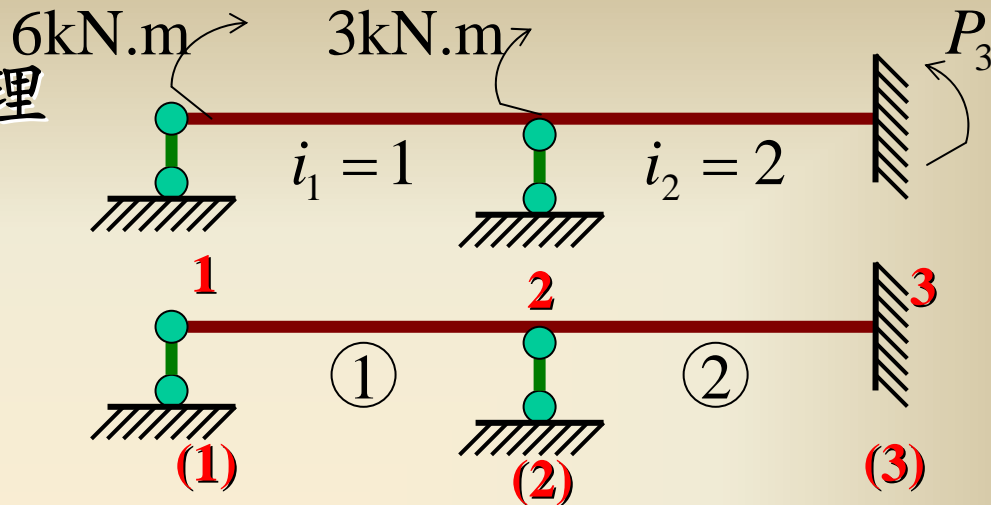
$$\begin{bmatrix} 1 & 0 & 0 \\ 0 & 12 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix} = \begin{Bmatrix} 0 \\ -3 \\ 0 \end{Bmatrix}$$



## 五.(零位移)边界条件处理

方法:  $\left\{ \begin{array}{l} \text{先处理法} \\ \text{后处理法} \end{array} \right.$

后处理法:  $\left\{ \begin{array}{l} \text{置0置1法} \\ \text{乘大数法} \end{array} \right.$



(1) 置0置1法

(2) 乘大数法

若  $\delta_i = 0$ , 则将总刚主对角元素  $k_{ii}$  乘以大数  $N$ .

$$\begin{bmatrix} 4 & 2 & 0 \\ 2 & 12 & 4 \\ 0 & 4 & 8 \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix} = \begin{Bmatrix} -6 \\ -3 \\ P_3 \end{Bmatrix}$$

$$\delta_3 = 0$$

$$\begin{bmatrix} 4 & 2 & 0 \\ 2 & 12 & 4 \\ 0 & 4 & 8 \times N \end{bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{Bmatrix} = \begin{Bmatrix} -6 \\ -3 \\ P_3 \end{Bmatrix}$$

第三个方程变为:

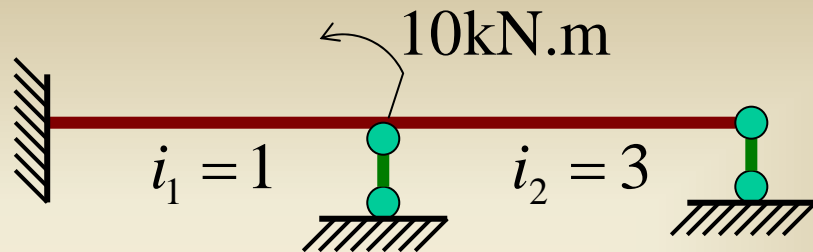
$$0 \times \delta_1 + 4 \times \delta_2 + 8 \times N \times \delta_3 = P_3$$

$$\delta_3 = (P_3 - 0 \times \delta_1 - 4 \times \delta_2) / (8 \times N)$$

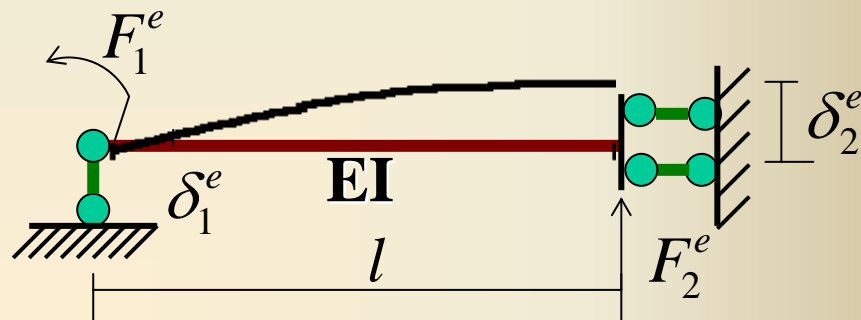
$$\delta_3 \approx 0$$

作业:

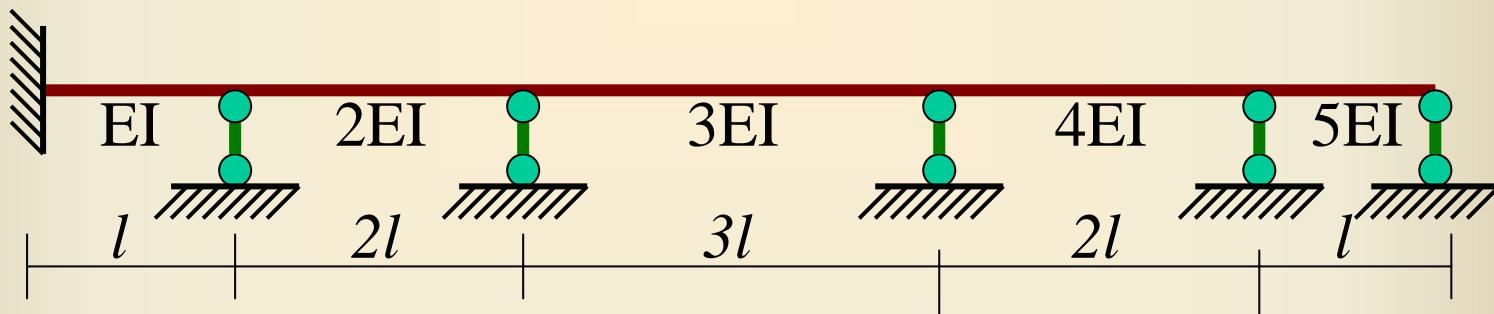
1. 作图示结构弯矩图



2. 推导图示单元的单刚



3. 计算图示梁总刚中元素  $k_{44}$   $k_{23}$   $k_{25}$



4. 思考题

- (1). 连续梁的总刚为何应是一个三对角矩阵?
- (2). 荷载不作用于结点上时怎么办?
- (3). 连续梁单刚和总刚是奇异还是非奇异矩阵?